

(1) calculate the quantity of earth work in cutting & banking for a portion of road for 120m length with the following data.

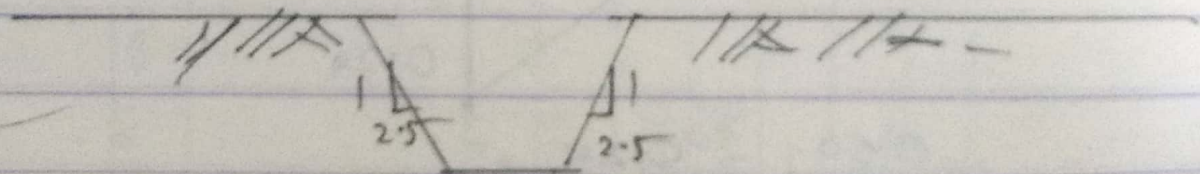
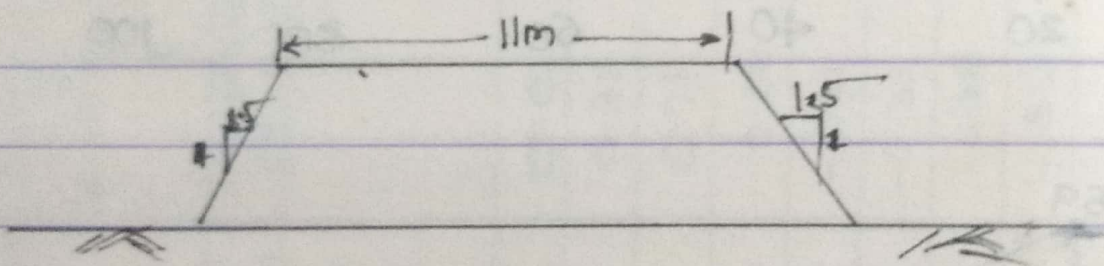
chainage (m)	0	20	40	60	80	100	120
G.L (m)	161.5	160.95	160.55	161.35	161.85	162.25	162.35

Formation width of road = 11m

Formation width of starting chainage = 161.5m

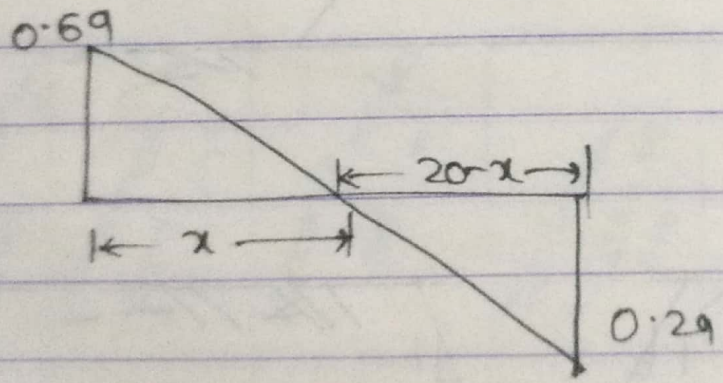
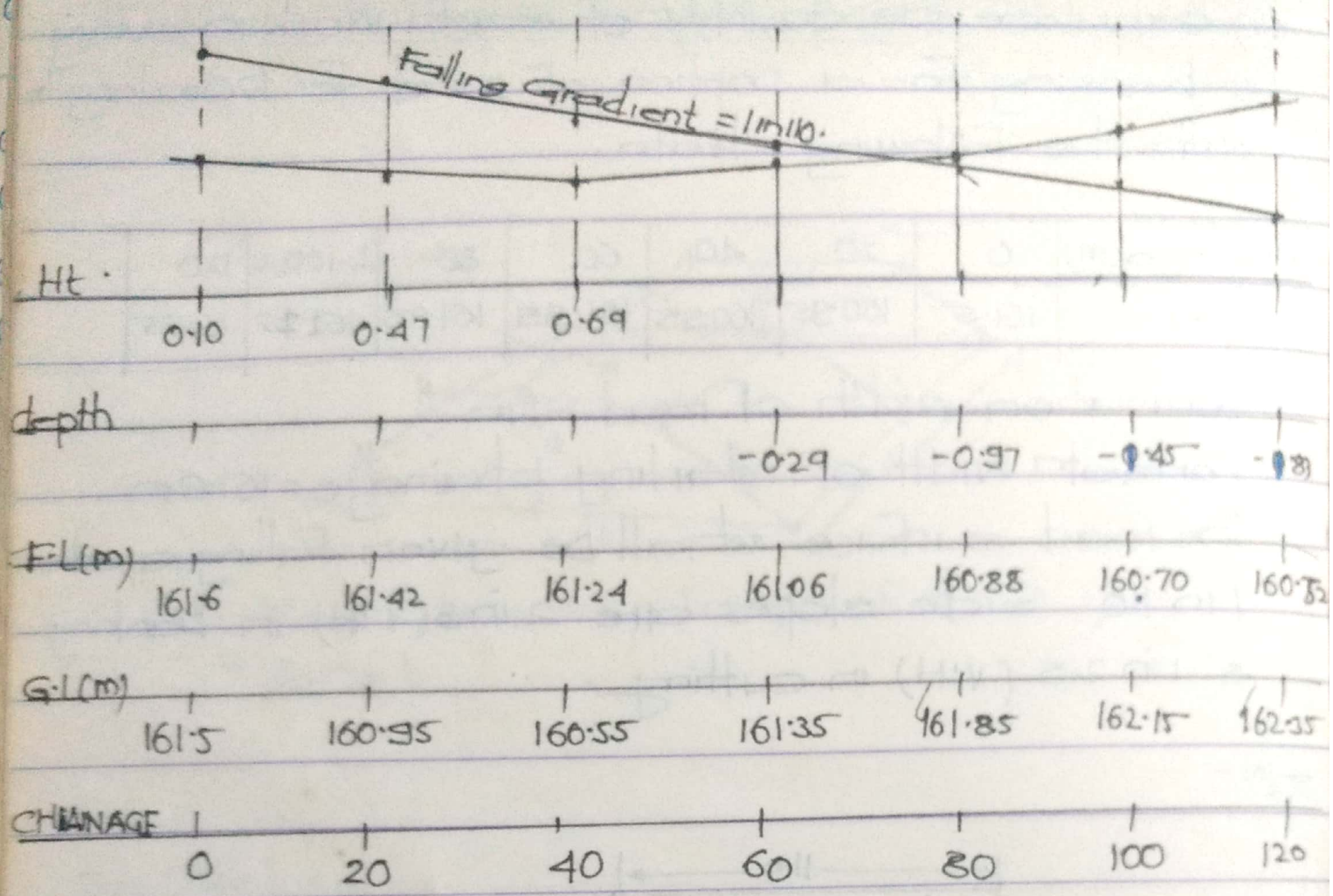
The road surface shall be given falling gradient 1 in 110. side slopes are 2 in 3 (V:H) in banking & 1 in 2.5 (V:H) in cutting.

50/m -



Falling Gradient of road = 1 in 110

∴ Gradient drop @ every 20m interval = $\frac{20}{110} = 0.18m$



$$\frac{0.69}{x} = \frac{0.29}{20-x}$$

$$13.80 - 0.69x = 0.29x$$

$$13.80 = 0.98x$$

$$\therefore x = 14.08 \text{ From L.H.S}$$

$$20 - 14.08 = 5.92 \text{ From R.H.S}$$

$B = 11m$
 Sin cutting = 2.5
 Sin Banking = 1.5

Changings	Hor depth (dm)	C/S area of C.P B x d (m ²)	Area of sides Sd ² (m ²)	Total area A = Bd + Sd ² (m ²)	Mean area Am = A1 + A2 / 2	Dist bett foot L (m)	Quantity of earthwork Banking Cutting
0	0.19 -	1.1	0.02	1.12	-	-	-
20	0.47 0.47	5.17	0.33	5.50	3.31	20	66.20
40	0.69 0.69	7.59	0.71	8.30	6.90	20	138.00
60 60	0.92 0.92	0.00	0.00	0.00	4.15	14.08	58.43
80	0.97	-3.19	-0.21	3.40	1.70	20 5.92	10.06
100	1.45 1.45	-10.67	-2.35	13.02	8.21	20	164.20
120	1.83	-15.95	-5.26	21.12	17.07	20	341.40
		-20.13	-8.37	28.50	24.81	20	496.20
							262.63
							1011.85

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(2) calculate the quantity of earth work in cutting & banking for a portion of road of 240m length with following data

changes (m)	0	40	80	120	160	200	240
G.L (m)	50.50	50.80	50.60	50.70	51.20	51.40	51.30

Formation level = 10m

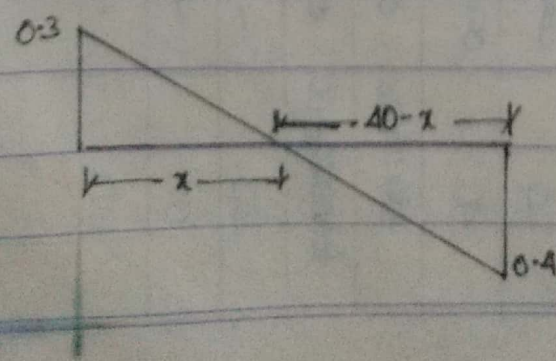
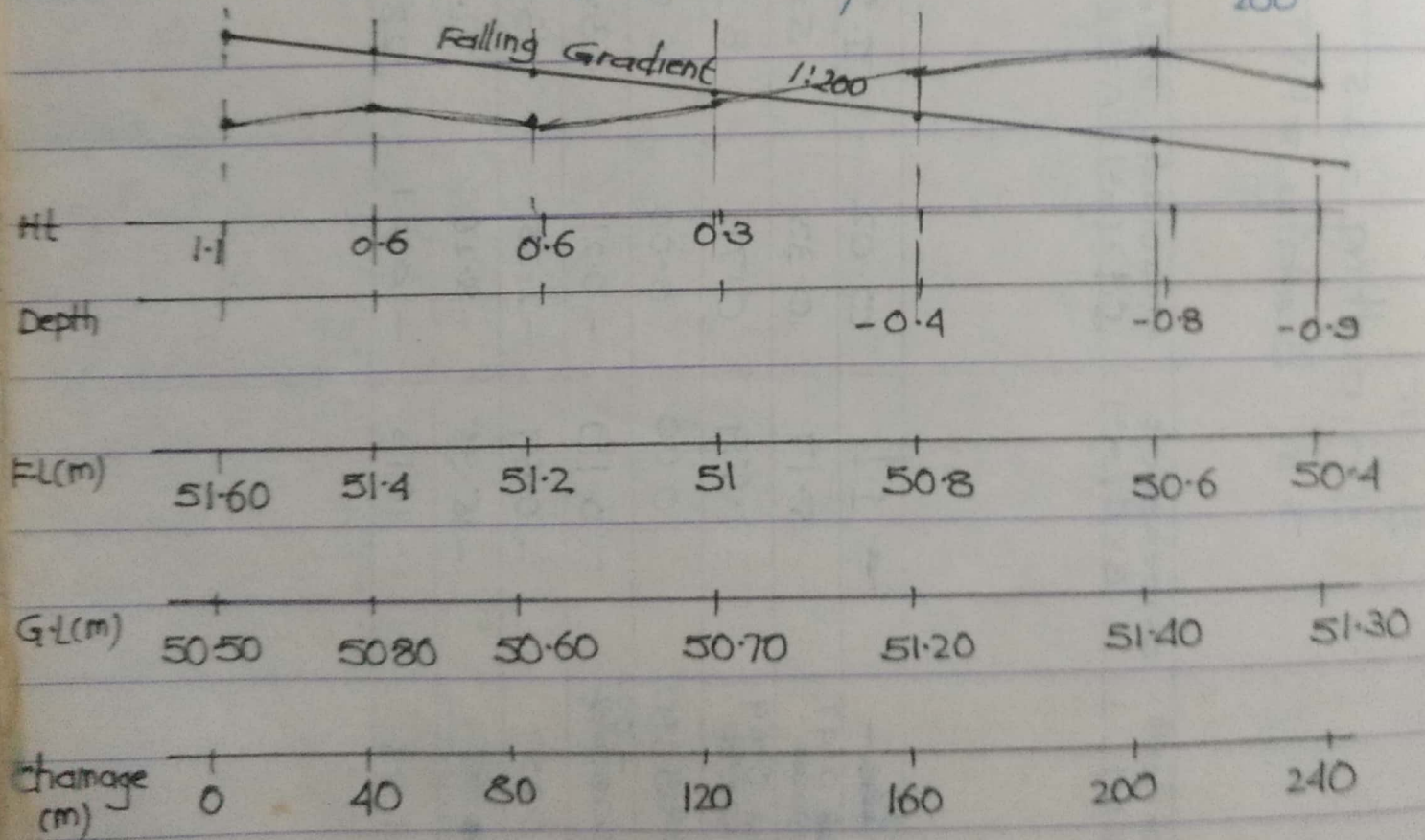
———— of starting chainage = 51.60m

Falling Gradient 1 in 200

steep Banking (2:1) & cutting (1.5:1)

Solⁿ:- Falling Gradient = 1 in 200

∴ Gradient drop @ every 40m interval = $\frac{40}{200} = 0.2m$



$$\frac{0.3}{x} = \frac{0.4}{40-x}$$

$$12 - 0.3x = 0.4x$$

$$\therefore x = 17.14m$$

$$\therefore 40 - x = 22.85m$$

B = 10m
 sin Banking = 2 sin cutting = 1.5

H.L.6

chainage (m)	Ht/depth (d)	B x d (m ²)	sd ² (m ²)	A = Bd + sd ² (m ²)	Am = L ₁ + L ₂ / 2	L (m)	Qty of earth work		
							BANKING	CUTTING	
0	1.1	11.00	2.42	13.42	—	—	—	—	
40	0.6	6.00	0.72	6.72	10.07	40	402.80	—	
80	0.6	6.00	0.72	6.72	6.72	40	268.80	—	
120	0.3	3.00	0.18	3.18	4.95	40	198.00	—	
PASSES	0.00	0.00	0.00	0.00	1.59	17.14	27.25	—	
160	-0.4	-4.00	0.24	4.24	2.12	22.85	—	48.44	
200	-0.8	-8.00	0.96	8.96	6.60	40	—	264.00	
240	-0.9	-9.00	1.22	10.22	9.59	40	—	383.60	
Total qty of earth work.							896.85	696.04	

Trapezoidal formula:

$$Q = \frac{L}{2} \left[\text{First area} + \text{last area} + 2 \left[\text{sum of middle areas} \right] \right]$$

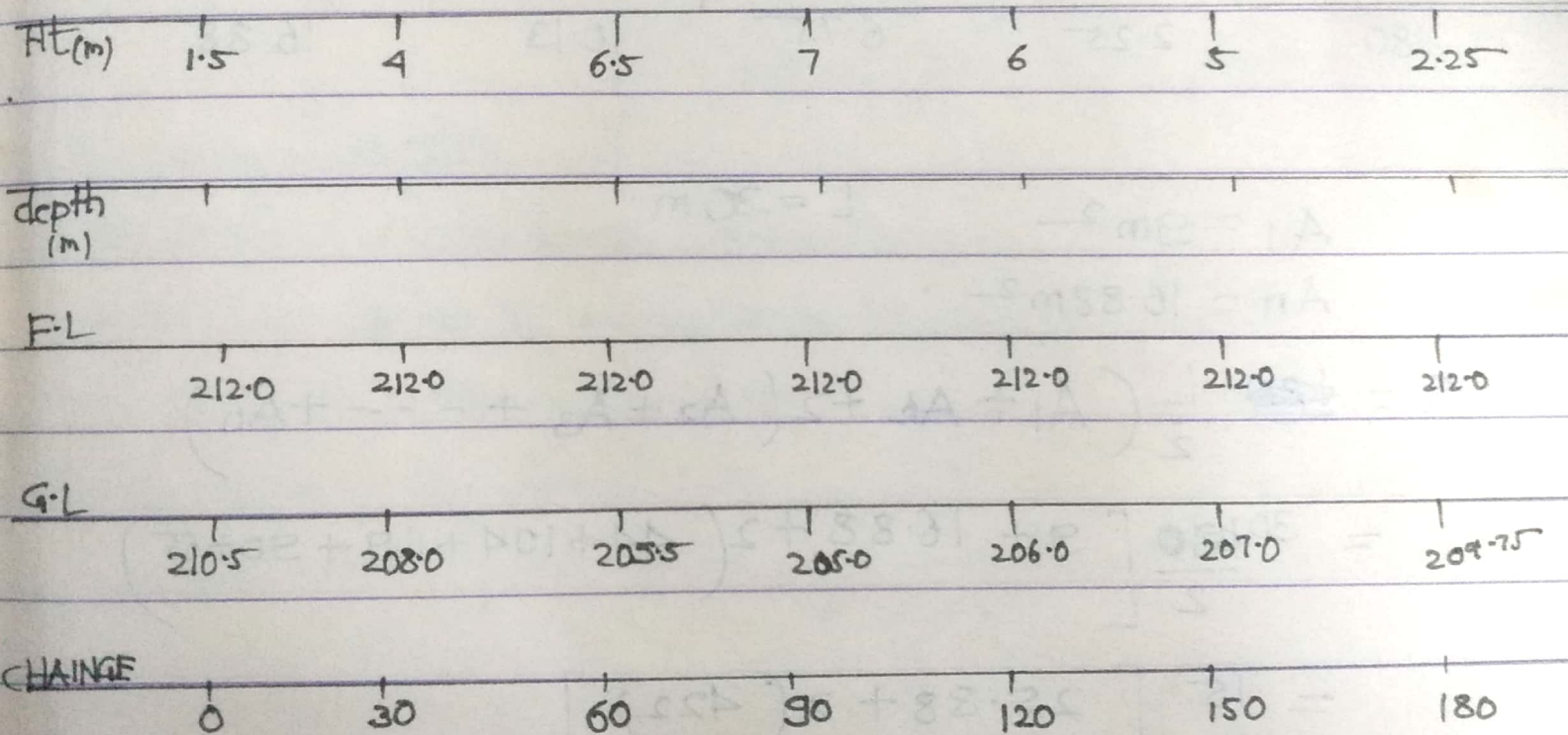
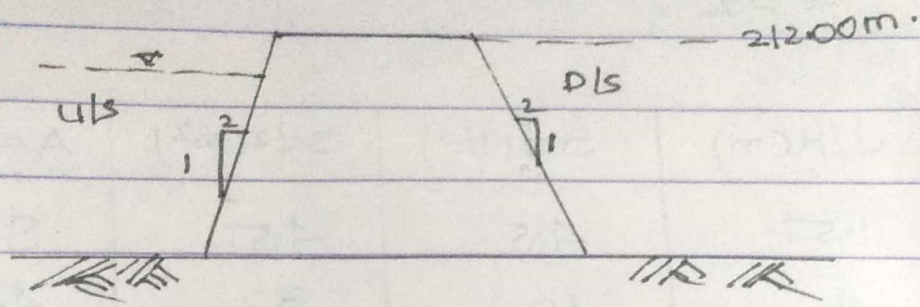
$$= \frac{L}{2} \left[A_1 + A_n + 2 \left[A_2 + A_3 + \dots + A_{n-1} \right] \right]$$

chainage	d(m)	Bd(m ²)	sd ² (m ²)	A = Bd + sd ² (m ²)

1) calculate the quantity of earth work for construction of a percolation tank with following data.

- (i) Top width = 3m
- (ii) R.L at top of embankment = ~~210~~ 212.0m
- (iii) side slope 2H:1V for upstream & down stream side
- (iv) G.L with chainage.

chainages	0	30	60	90	120	150	180
G.L	210.5	208.0	205.5	205.0	206.0	207.0	209.75



$$B = 3m$$

$$S = 2$$

chainage	d/H(m)	Bd (m ²)	Sd^2 (m ²)	$A = Bd + Sd^2$ (m ²)
0	1.5	4.5	4.5	9
30	4	12	32	44
60	6.5	19.5	84.50	104
90	7	21	98	119
120	6	18	72	90
150	5	15	50	65
180	2.25	6.75	10.13	16.88

$$A_1 = 9m^2$$

$$L = 30m$$

$$A_n = 16.88m^2$$

$$Q = \frac{L}{2} (A_1 + A_n + 2(A_2 + A_3 + \dots + A_n))$$

$$= \frac{30}{2} [9 + 16.88 + 2(44 + 104 + 119 + 90 + 65)]$$

$$= 15 [25.88 + 2(422)]$$

$$= 15 [25.88 + 844]$$

$$= 15 (869.88)$$

$$Q = 13.04 \times 10^3 m^3$$

\therefore Qty of earth work in embankment $= 13.04 \times 10^3 m^3$

RATE ANALYSIS

Market rate

- (1) Brick (Ist class, IInd class)
- (2) Stones (Rubble)
- (3) cement
- (4) sand
- (5) Aggregate (2 1/2, 1 1/2, 0.75, 1, 1 1/2) inches.
- (6) steel rate (Fe250, Fe415)
- (7) Door window frame (T.W)
- (8) Distemper, stone paint (water bound, oil bound)
- (9) Tiles (Marble, kota stones, Glazed tile, manolyas, ridge rate)
- (10) centering with material charges
- (11) Mason rate
- (12) Painter ——— (Kulre)
- (13) Bar bender ———
- (14) Carpenter, centering ———
- (15) Tile labours
- (16) Plumbers ———
- (17) Head Mason ———
- (18) Bisti
- (19) Made kadie
- (20) Female ———

$$1\text{m} = 3.28 \text{ feet}$$

$$1\text{m}^2 = 10.764 \text{ sq feet}$$

$$1\text{m}^3 = (3.28)^3 = 35.23 \text{ cubic feet}$$

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Market rates

Rs:- 1200/- per 1000 bricks

Rs:- 1500/- per Truck . :- 250/- per cubic meter

Rs:- 120/- per bags . :-

Rs:- 1800/- per truck . :- 300/- per m^3

Rs:- 2800/- per truck . 46 475/- per m^3

Rs:- 18/50/- per kg . 1850/- per quintal

Rs:- 1250/- per sq m .

Rs:- 400/- per sq feet . 4304 ~~3700~~ Rupees per m^2

(6700 Rupees) Marble :- 65/- per sq feet , cota 18/- per sq feet (19500 m^2)

Glazed tile :- 16/- per sq feet Marble 3/50/- sq feet

(172 Rupees m^2) Centering 5/- per wood . & 7/- per ply wood (30 Rupees m^2)

Rs :- 100/- to 180/- per sq feet

Rs :- 60/- to 80/- — " —

Rs :- 2/60, 2/50 per sq feet

Rs :- 100 / 180 / per — " —

Rs :- 4/50 — " —

Rs :- 80 to 150 per — " —

Rs :- 80 to 150 per — " —

Rs :- 60/- per day — " —

Rs :- 70/- per day } 8 hrs

Rs :- 50/- per day }

Rate analysis:

The method of determining the rate of particular item of work considering the quantities & cost of material & labour required is called rate analysis.

Purpose:

- (1) To determine current rate per unit an item
- (2) To examine the viability of rate offered by contractor.
- (3) To calculate quantity of material & labour require for project planning
- (4) To fix up the labour contractor rate.

Factor affecting rate analysis:

- | | Total cost of work |
|---|--------------------|
| (1) <u>Major factor:</u> - (1) Material | (50% to 60%) |
| (2) Labour | (25% to 30%) |
| (2) <u>Minor Factors:</u> - (1) Tools & plant | (1.5 to 2%) |
| (2) Water charges | (2 to 3%) |
| (3) overhead charges | (2.5% to 3%) |
| (4) contractor's profit | (10%) |

Task work:

The quantity of work that can be done by labour in one day (i.e. 8 hrs) is called task work.

Factors affecting task work.

- (1) output of an individual worker varies considerably as per their skill.
- (2) The task work were varies from place to place as per climatic condition & environment of place.
- (3) Faulty work organization & ill planning.
- (4) Peculiarities of job & site condition may have some effect on task work.

Importance of rate analysis & requirement:-

It gives a clear picture of various type of labour & material required for completing a particular job.

Requirement:-

- (1) current market rate of material.
- (2) current market rate of various categories of
- (3) Task work.
- (4) Hire charges & information of various types of plants required for the work.
- (5) upto date knowledge of construction work.

Resources affect rate analysis

- (1) specification.
- (2) Quality of material
- (3) location of site.
- (4) Availability of tools & plant of work site
- (5) Profit & overhead expenses of contractor.

SRNO	Name of works	Task work
(1)	Excavation in black cotton soil	3m ³ / Mazdoor.
(2)	Excavation in Murum	2m ³ / Mazdoor.
(3)	Excavation in Rocky soil	1m ³ / Mazdoor.
(4)	Brick work in foundation & plinth	20m ³ / Head Mason.
		1.25m ³ / Mason.
		1.25m ³ / Male coolie.
		1.25m ³ / Female coolie.
		5m ³ / Bhisti.
(5)	Brick work in superstructure	20m ³ / Head Mason.
		1.0m ³ / Mason.
		1.0m ³ / Male coolie.
		1.0m ³ / Female coolie.
		5.0m ³ / Bhisti.
(6)	Half brick work. (partition wall)	5m ² / Mason.
		5m ² / Male coolie.
		5m ² / Female coolie.
		30m ² / Bhisti.

100m²

SNO	Name of work	Task work
(7)	U.C.R Masonry in plinth & foundation in C.M.	$20\text{m}^3 / \text{HM}$ $1\text{m}^3 / \text{M}$ $1.1\text{m}^3 / \text{MC}$ $1.1\text{m}^3 / \text{FC}$ $6.67\text{m}^3 / \text{Bhisti}$
(8)	C.R.M in plinth & foundation	$20\text{m}^3 / \text{HM}$ $0.8\text{m}^3 / \text{M}$ $1.1\text{m}^3 / \text{M.C}$ $1.1\text{m}^3 / \text{F.C}$ $6.67\text{m}^3 / \text{Bhisti}$
(9)	U.CRM in super structure	$20\text{m}^3 / \text{HM}$ $0.83\text{m}^3 / \text{M}$ $1\text{m}^3 / \text{MC}$ $1\text{m}^3 / \text{FC}$ $6.67\text{m}^3 / \text{Bhisti}$
(10)	CRM in super structure	$20\text{m}^3 / \text{H.M}$ $0.70\text{m}^3 / \text{M}$ $0.90\text{m}^3 / \text{MC}$ $0.90\text{m}^3 / \text{FC}$ $6.67\text{m}^3 / \text{Bhisti}$
(11)	Ashlar masonry	$20\text{m}^3 / \text{HM}$ $0.4\text{m}^3 / \text{M}$ $0.6\text{m}^3 / \text{MC}$ $0.6\text{m}^3 / \text{FC}$ $6.67\text{m}^3 / \text{Bhisti}$

SNO	Name of work	Estimate work ✓
(12)	D.C.C in foundation	40 m ³ / H.M 5 m ³ / M 2 m ³ / M.C, 2 m ³ / F.C 5 m ³ / Bhisti.
(13)	^{Concrete} D.C.C for R.C.C work	20 m ³ / H.M 3 m ³ / M 1.10 m ³ / M.C 1.10 m ³ / F.C 2.5 m ³ / Bhisti.
(14)	25mm th cement concrete D.P.C	200 m ² / H.M 12.5 m ² / M 12.5 m ² / Majdoor 100 m ² / Bhisti.
(15)	25mm th ^{cement concrete} D.C.C flooring	200 m ² / H.M 12.5 m ² / M 8 m ² / majdoor 50 m ² / Bhisti.
(16)	2mm th cement plaster	200 m ² / H.M 10 m ² / M 7 m ² / majdoor 100 m ² / Bhisti.
(17)	Pointing	200 m ² / H.M 10 m ² / M 10 m ² / majdoor 100 m ² / Bhisti.

SNO	Name of work.	Task Work.
(18)	centering & shuttering.	3.25 m ² / carpenter 2.50 m ² / coolie
(19)	Reinforcement	1 qunit / Bar bender 1 qunit / helper
(20)	White wash or colour wash (3 coats)	200 m ² / painter
(21)	Distemper	200 m ² / coolie 40 m ² / painter
(22)	Lime punning	40 m ² / coolie 10 m ² / M.
(23)	Mangalore type roofing with battens	5 m ² / coolie 10 m ² / Carpenter 6 m ² / tile labourer 5 m ² / coolie

Material requirement for different C.F works:-

- (1) Brick 500 nos / m³.
- (2) cement 30 bags / m³ (vol^m of 1 bag - 0.035 m³)
- (3) Lime Burning.

SNO	Item	Materials	Quantity
(1)	Brick	Brick	500 no/m ²
(2)	cement	cement	30 bags/m ²
(3)	lime punning	lime stone	1.8 kg/m ²
(4)	white wash.	lime stone	10 kg/100m ²
(5)	white wash 2 coats	lime stone	30 kg/100m ²
(6)	Distempering 1st coat. 11rd coat	Distemper	12 kg/100m ² 7.5 kg/100m ²
(7)	Reinforcement	Binding wire	1kg/ quintal of steel

Dry vol^m of lime = 44% of wet vol^m.

UCR Masonary: — Dry vol^m of CM = 42% of wet vol^m.

CR Masonary: — Dry vol^m of CM = 40% of wet vol^m.

Ashtlar Masonary: — Dry vol^m of CM = 25% of wet vol^m.

concrete: — Dry vol^m of CM = 52% of wet vol^m.

Brick work: — Dry vol^m of CM = 30% to 35% — "

pointing: — (a) Flush pointing — Dry vol^m = 0.7m³/100m²

(b) Roled pointing — Dry vol^m = 0.63m³/100m²

Q1) Prepare the rate analysis for excavation for foundation in B.C soil.

Soln - Excavation for foundation in B.C soil :-
considered 10m^3 of excavation

SNO	Description	Quantity	Rate		Per unit	Amount	
			Rs	Pc		Rs	Pc
(1)	<u>Labour</u> :-						
	Male coolie	2 Nos	70	00	per day	140	00
	F. coolie	$1\frac{1}{2}$ Nos	50	00	per day	75	00
(2)	<u>Secondaries</u> :- (including hire charges)	Lumpsum				3	00
					Total	218	00
						21	80
					Grand total	239	80

Add contractor profit (10)%

$$\therefore \text{The rate of excavation/m}^3 = \frac{239.80}{10}$$

$$= 23.98$$

$$= 24 \text{ Rs}$$

Excavation for foundation in Murum soil :-
considered 10m² of excavation :-

SNO	Description	Quantity	Rate	Per unit	Amount
		Rs m ³	Rs pc		Rs pc
(1)	<u>Labour</u>				
	M-C	3 Nos	70.00	per day	210 - 00
	F-C	2 Nos	50.00	per day	100 - 00
(2)	sundaries (including hire charges) (10%)	Lumpsum			3 - 00

∴ Total = 313 - 00

Add contractor profit (10%)

31 - 30

∴ Grand total = 344 - 30

∴ The rate of excavation/m³ = $\frac{344.30}{10}$

= 34.43

≈ 35.

(2) Prepare rate analysis of IInd class B.B masonry for plinth & foundation.

Solⁿ - IInd B.B masonry in C.M (1:6) plinth & foundation considered 10m³ of B.B masonry :-

For dry vol^m of C.M increase wet vol^m by 35%

$$= 2.65 + \frac{35}{100} \times 2.65$$

$$= 3.58 \text{ m}^3$$

$$\boxed{\text{Dry vol}^m \approx 3.6 \text{ m}^3}$$

$$\text{Cement} = \frac{\text{Total vol}^m}{\text{sum of ratio of CM}} \times \text{Representative of cement}$$

$$= \frac{3.6}{(1+6)} \times 1$$

$$= 0.51 \text{ m}^3$$

$$= 0.15 \times \text{Bag of cement}$$

$$= 0.15 \times 36$$

$$= 5.43$$

$$\boxed{\text{Cement} \approx 15 \frac{1}{2} \text{ bags of cement}}$$

$$\text{Sand} = \frac{\text{Total dry vol}^m}{\text{sum of ratio of CM}} \times 6$$

$$= \frac{3.6}{(1+6)} \times 6$$

$$= 3.09 \text{ m}^3$$

$$\boxed{\text{Sand} \approx 3 \text{ m}^3}$$

considered 10m³

unit cum

SNO	Description	Qty	Rate		per unit	Amount		
			Rs	Ps		Rs	Ps	
(A) Materials:-								
	(1) Bricks	5000 Nos	1200	00	per 1000 Nos	6000	00	
	(2) cement	15 1/2	120	00	per bag	1860	00	
	(3) sand	30	300	00	m ³	900	00	
(B) Labour:-								
	H.M	1/2 Nos	150	00	per day	75	00	
	M	8 Nos	100	00	per day	800	00	
	M.C	8 Nos	70	00	per day	560	00	
	F.C	8 Nos	50	00	"	400	00	
	Bhisti	2 Nos	60	00	"	120	00	
(C) Sundaries:-								
(Including fire charge) Lumpsum (3%)						300	00	
						Total =	11015	00
(d) a.p Add 2% water charges							220	30
Add 10% c.p							1101	50
Grand total							12336	80

: Rate of 1st class B.B masonry per m³

$$= \frac{12336.80}{10}$$

$$= 1233.68$$

$$Rs \approx 1234.1 - m^3 / -$$

(3) Prepare a rate analysis of 1st class B.B masonry for superstructure.

Soln:- considered 10m³ of B.B masonry.
second class B.B masonry in CM(1:3) for superstructure.

(1) Materials (a) Bricks:-

Nominal size = (0.19 x 0.09 x 0.09)

std size = (0.2 x 0.1 x 0.1)

∴ No of bricks = $\frac{10}{0.2 \times 0.1 \times 0.1}$

∴ No of bricks = 5000 Nos

(b) cement Mortar:- $10 - (5000 \times 0.19 \times 0.09 \times 0.09)$
= 2.3m³

Increase 15% of ^{wet} vol^m for frog filling & other wastage

= $2.3 + \frac{15}{100} \times 2.3$

= 2.65m³

For dry vol^m of CM increase wet vol^m by 35%

= $2.65 + \frac{35}{100} \times 2.65 = 3.6m^3$

(3) cement:- $\frac{36}{1+3} \times 1 = 0.9m^3$

= 0.9 x 30

= 27 bags of cement

(4) sand:- $\frac{3.6}{1+3} \times 3 = 2.7m^3$

considered 10m³ in cum

SNO	Description	Qty	Rate		Per unit	Amount	
			Rs	Paise		Rs	Paise
(1)	<u>Materials:-</u>						
	(a) Bricks	5000	4100 1200	00	Per 10 ³ Nos	6000	00
	(b) cement	27	120 530	00	per bags	3240	00
	(c) sand	2.7	300 550	00	m ³	810	00
(2)	<u>labours:-</u>						
	H-M	1/2 Nos	150 650	00	Per day	75	00
	M	10 NOS	100 560	00	— —	1000	00
	M-C	10 NOS	70 300	00	— —	700	00
	F-C	10 Nos	50 200	00	— —	500	00
	B	2 NOS	60 200	00	— —	120	00
(3)	Sundaries (Including hire charges) lumsun (3%)					300	00
					Total	12745	00
(4)	Add 2% of water charges					254	90
	Add 10% of CP					1274	50
					GRAND Total =	14274	40

∴ Rate of IInd class B.B Masonary for super structure

per m³ = $14274.40 / 10 = 1427.44$

∴ Rs = 1428/-

(4) Prepared a rate analysis for half brick work.

solⁿ: considered 10m³ of partition wall.

Materials:- (a) Bricks = 5000 Nos.

(b) cement = 27 bags of cement.

(c) sand = 2.7m³.

SNO	Description	Qty	Rate		Per unit	Amount	
			Rs	ps		Rs	ps
(1)	Materials	5000					
	Bricks	5000	1200	00	per 1000 nos	6000	00
	cement	27	120	00	per bag	3240	00
	sand	2.7	300	00	per m ³	810	00

(2) Labours

M	2 Nos	150	00	per day	200	00
MC	2 Nos	700	00	—	1400	00
FC	2 Nos	50	00	—	100	00
B	1/3 Nos	60	00	—	20	00

(3) sundaries (Add 1% (2%) of Hire charge 240 00

Total | 12010 00

Add 2% of water charges = 240 20

Add 10% of CP = 1201 00

GRAND TOTAL 13451 20

∴ Rate of partition wall in / m³ = $\frac{13451.20}{10} = 1345.12$ /-

(5) Prepare a rate analysis for Ashlar masonry in foundation in CM (1:6)

Soln:- considered 10m^3 of Ashlar masonry.

Materials:- (i) Ashlar masonry = 12.5m^3

(ii) cement = 11 bags.

(iii) sand = 2.14m^3

SNO	Description	Qty	Rate		per unit	Amount	
			Rs	Ps		Rs	Ps.
(1) Materials							
	Ashlar masonry	12.5	250	00	per m^3	3125	00
	Cement	11	120	00	per bags	1320	00
	Sand	2.14	300	00	per m^3	642	00
(2) Labour							
	H-M	1/2 Nos	150	00	per day	75	00
	M	23 Nos	100	00	—	2300	00
	MC	17 Nos	70	00	—	1190	00
	FC	17 Nos	50	00	—	850	00
	B	1 1/2 Nos	60	00	—	90	00
(3) sundaries (Add 3%)						293	76
Total =						1085	76
Add 2% of water charges =						20	71
Add 10% of CP =						108	57
GRAND TOTAL						11296	04

∴ Rate of Ashlar masonry in

$$\text{foundation / m}^3 = \frac{11296.04}{10} = \text{Rs} = 1129.60$$

(5) Prepare a rate analysis for u.c.r masonry in foundation in CM (1:6)

solⁿ:- considered 10m^3 of u.c.r masonry

Materials:-

(1) Rubble / stone

Qty of stone = 10m^3

Add 25% for wastage.

$$\begin{aligned}\therefore \text{Qty required} &= 10 + \frac{15}{100} \times 10 \\ &= 12.5\text{m}^3\end{aligned}$$

(2) cement mortar:-

Dry of CM required = 42% of 10m^3 considered

$$= \frac{42}{100} \times 10$$

$$= 4.2\text{m}^3$$

$$(3) \text{ cement} = \frac{4.2}{1+6} \times 1$$

$$= 0.6\text{m}^3$$

$$\therefore \text{No of bags} = 0.6 \times 30$$

$$= 18 \text{ bags}$$

$$(4) \text{ sand} = \frac{4.2}{1+6} \times 6 = 3.6\text{m}^3$$

SNO	Description	Qty	Rate		per unit	Amount	
			Rs	Pse.		Rs	Ps
(1)	Materials:						
(a)	Rubbl/stone	12.5	250	00	per m ³	3125	00
(b)	cement	18	120	00	per bags	2160	00
(c)	sand	3.6	300	00	per m ³	1080	00
(2)	labours:-						
	H.M	1/2 Nos	150	00	per day	75	00
	M	10 Nos	100	00	- -	1000	00
	M.C	9 Nos	70	00	- -	630	00
	F.C	9 Nos	50	00	- -	450	00
	B	1.5 Nos	60	00	- -	90	00

(3) sundaries

(including hire charges) $\frac{1}{10}$ sum 311 210 00

(4)	Add 1.5% of water charges =	Total	7161 - 00
	Add 10% of ep		107 - 41
			76 - 10
		Total	7984 - 51

\therefore Rate of U.C.R Masonary for plinth & foundation per m³ = $\frac{7984.51}{10} = 800$

(6) Prepare a rate analysis for P.C.C (1:4:8) in foundation.

Soln:- considered 1 m^3 for P.C.C (1:4:8) in foundation.

Materials :-

(1) concrete :-

wet vol^m of concrete = 1 m^3

For dry vol^m increase vol^m by 52%.

$$\therefore \text{Dry vol}^m \text{ of concrete} = 101 \frac{52}{100} \times 10 = 15.2 \text{ m}^3.$$

(2) cement = $\frac{15.2}{1+4+8} \times 1$

$$= 1.16 \text{ m}^3$$

\therefore No of bags = $1.16 \times 30 = 35$ bags.

(3) Sand = $\frac{15.2}{1+4+8} \times 4$

$$= 4.67 \text{ m}^3$$

(4) Agg = $\frac{15.2}{13} \times 8$

$$= 9.35 \text{ m}^3$$

SNO	Description	Qty	Rate		Per unit	Amount	
			Rs	Ps		Rs	Ps
(1)	<u>Materials</u>						
(a)	concrete cement	152.22 35	120	00	per bags	4200	00
(b)	sand	4.67	300	00	per m ³	1401	00
(c)	Agg	9.35	475	00	per m ³	4441	25
(2)	<u>labour</u>						
	H.M	1/4 No	150	00	per day	37	50
	M	2 No	100	00	- -	200	00
	B	2 No	60	00	- -	120	00
	M-C	5 No	70	00	- -	350	00
	F-C	5 No	50	00	- -	250	00
(3)	sundaries (Add 4%)					450	00
Total						11449	75

(4)	Add 2% of WC	228	99
(5)	Add 10% of CP	1144	97
Grand total		12823	71

∴ Rate of P.C.C (1:4:8) in foundation in /m³ = $\frac{12823.71}{10}$
 RS = 12823/-

(7) Prepare a rate analysis for R.C.C. for slab (1:2:4)
slab considered 10m^3 for R.C.C. for slab (1:2:4)

Materials:-

(1) concreting:- Net vol^m of conct = 10m^3

For dry vol^m increase vol^m by = 52%.

$$= 15.2\text{m}^3$$

(2) Cement = $\frac{15.2}{(1+2+4)} \times 1 = 2.17 \times 30 = 65\text{bags}$

(3) sand = $\frac{15.2}{(1+2+4)} \times 2 = 4.34\text{m}^3$

(4) Agg = $\frac{15.2}{(1+2+4)} \times 4 = 8.68\text{m}^3$

(5) Reinforcement = $\frac{1}{100}$ of ^{net} vol^m of concrete

$$= \frac{1}{100} \times 10 \times 18.5$$

$$= 7.85 \text{ Quintals}$$

(6) Binding wires = 8 kg ✓

(7) centering = ~~3000~~ 100m^2

considered $10m^3$.

SNO	Description	Qty	Rate		per unit	Amount	
			Rs	P5		Rs	P5
<u>(1) Materials:-</u>							
	cement	65	120	00	per bag	7800	00
	sand	4.34	300	00	per m^3	1302	00
	Agg	8.68	475	00	per m^3	4123	00
	Reinforcement	7.85	1850	00	per qts.	14522	50
	Binding wires	8	30	00	per kg	240	00
	centering	100	30	00	per m^2	3000	00
<u>(2)</u>							
	Labour for concrete	0.5 Nos	150	00	per day	75	00
	H.M	3.5 Nos					
	M,	3.5 Nos	100	00	per day	350	00
	M.C	9 Nos	70	00	per day	630	00
	F.C	9 Nos	50	00	per day	450	00
	Bristle	4 Nos	60	00	per day	240	00
<u>(3)</u>							
	Labour for centering	30 Nos	100	00	per day	3000	00
	Barbender						
	coolie	40 No	70	00	per day	2800	00
<u>(4)</u>							
	Labour for centering						
	Barbender	8	120	00	per day	960	00
	helper	8	70	00	per day	560	00

(c) scundaries (Add 5%) \longrightarrow $\begin{array}{r|l} 4005 \cdot 2 & 50 \\ 2002 & 60 \end{array}$

Total = 42054 ~~40~~ 50

\therefore Add water charges (1.5%) = $630 \cdot 81$

Add C.P 10% = $4205 \cdot 45$

GRAND Total = $46890 \cdot 76$

\therefore Rate of R.C.C for slab (1:2:4) / $m^2 = \frac{46890 \cdot 76}{10}$

$\boxed{Rs = 4689/-}$
 $\boxed{Rs \approx 4700/-}$

(8) Prepare a Rate analysis for D.P.C (1:2:4)

So/n:- considered 100m^2 .

$$\begin{aligned}\therefore \text{Wet vol}^m \text{ of concrete} &= \text{Area} \times \text{thickness} \\ &= 100 \times 0.025 \\ &= 2.5\text{m}^3\end{aligned}$$

$$\begin{aligned}\therefore \text{Dry vol}^m \text{ by } 52\% \text{ Add} &= 2.5 + \frac{52}{100} \times 2.5 \\ &= 3.8\text{m}^3\end{aligned}$$

$$\begin{aligned}\text{(a) } \therefore \text{Cement} &= \frac{3.8}{(1+2+4)} \times 1 \\ &= ~~0.5~~ 16 \text{ bags}\end{aligned}$$

$$\begin{aligned}\text{(b) } \text{Sand} &= \frac{3.8}{7} \times 2 \\ &= 1.08\text{m}^3\end{aligned}$$

$$\begin{aligned}\text{(c) } \text{Agg} &= \frac{3.8}{7} \times 4 \\ &= 2.17\text{m}^3\end{aligned}$$

$$\begin{aligned}\text{(d) } \text{Water proofing compound} &= 2 \text{ to } 3\% \text{ by wt of cement} \\ &= \frac{2}{100} (\text{No of c.B} \times \text{wt of one c.B}) \\ &= \frac{2}{100} \times (16 \times 30) \\ &= 16 \text{ kg.}\end{aligned}$$

(g) Prepared a rate analysis for 25mm thick cement concrete flooring (1:2:4)
soln:- considered 100m²

$$\begin{aligned}\therefore \text{Net vol}^m \text{ of concrete} &= \text{Area} \times \text{thickness} \\ &= 100 \times 0.025 \\ &= 2.5 \text{m}^3\end{aligned}$$

increase dry vol^m by 52%

$$\begin{aligned}&= 2.5 + \frac{52}{100} \times 2.5 \\ &= 3.8 \text{m}^3\end{aligned}$$

$$\begin{aligned}\text{(a) cement} &:- \frac{3.8}{(1+2+4)} \times 1 \\ &= 16 \text{ bags}\end{aligned}$$

$$\begin{aligned}\text{(b) sand} &:- \frac{3.8}{(1+2+4)} \times 2 \\ &= 1.08 \text{m}^3\end{aligned}$$

$$\begin{aligned}\text{(c) Agg} &= \frac{3.8}{7} \times 4 \\ &= 2.17 \text{m}^3\end{aligned}$$

considered 100m²

SNO	Description	Qty	Rate		per	Amount	
			Rs	Ps		Rs	Ps
(1)	<u>Materials</u>						
	Cement	16	120	00	per bags	1920	00
	Sand	1.08	300	00	per m ³	324	00
	Agg's	2.17	475	00	per m ³	1030	75
(2)	<u>Labours</u>						
	H.M	1/2 Nos	150	00	per day	75	00
	M	8 Nos	100	00	"	800	00
	<u>Mazdoor</u>						
(a)	M	5 Nos	70	00	"	350	00
(b)	F	7 1/2 Nos	50	00	"	375	00
	B	2 Nos	60	00	"	120	00
(3)	<u>Surplaries</u>	(Add 3%)			=	150	00
					Total =	5144	75
(4)	Add 2% of water charges					102	89
(5)	Add 10% of contractor profits					514	47
					GRAND TOTAL =	5762	11

∴ Rate of 25mmth cement concrete

$$\text{flooring / m}^2 = \frac{5762.11}{100} = 57.62$$

(Q) prepared a rate analysis for cement plaster in CM (1:4)

solⁿ:- considered 100m².

Materials:-

$$\text{Area of plaster} = 100 \text{ m}^2$$

$$\begin{aligned} \text{Qty of cement mortar} &= 100 \times 0.012 \\ &= 1.2 \text{ m}^3 \end{aligned}$$

increase wet vol^m by 20 to 25 % for uneven surface & wastage

$$\text{wet of C.M} = 1.2 + \frac{20}{100} \times 1.2$$

$$= 1.44 \text{ m}^3$$

For dry vol^m of CM increasing wet vol^m by 35%

$$\text{Dry vol}^m \text{ of C.M} = 1.44 + \frac{35}{100} \times 1.44$$

$$= 1.94 \text{ m}^3$$

$$\approx 2 \text{ m}^3$$

$$\text{cement} = \frac{2}{(1+4)} \times 1$$

$$= 12 \text{ bags}$$

$$\text{sand} = \frac{2}{(1+4)} \times 4$$

$$= 1.6 \text{ m}^3$$

(1:4)

SNO	Description	Qty	Rate		Per	Amount	
			Rs	Ps		Rs	Pc
(1)	<u>Materials</u>						
	cement	12	120	00	per bag	1440	00
	sand	1.6	300	00	per m ²	480	00
(2)	<u>Labour</u>						
	H.M	1/2 Nos	150	00	per day	75	00
	M	10 Nos	100	00	— —	1000	00
	Majdoor	15 Nos	60	00	— —	870	00
	Bhisti	1 Nos	60	00	— —	60	00
(3)	sundaries (add 4%)					156	02
					Total	4082	00
					Add water charges (2%)	=	81 64
					Add C.P (10%)	=	408 2
					GRAND Total	=	4571 84

∴ Rate of 12mm thick cement plaster/m² = $\frac{4571.84}{1}$

Majdoor	15						
M	5	70				350	00
F.	10	50				300	
						15	
						4061	2

(Q) Prepare rate analysis for Neer plaster in CM(1:4)

soln:- considered 100m^2

Materials:- Area of plaster = 100m^2

$$\text{Qty of cement M} = 100 \times 0.012 \\ = 1.2\text{m}^3$$

$$\text{Wet vol}^m \text{ of CM} = 1.2 + \frac{20}{100} \times 1.2 \\ = 1.44\text{m}^3$$

$$\text{Dry vol}^m \text{ of CM} = 1.44 + \frac{35}{100} \times 1.44 \\ = 1.94\text{m}^3 \\ \approx 2\text{m}^3$$

$$(1) \text{ cement} = \frac{2}{1+4} \times 1 \\ = 12 \text{ bags}$$

$$(2) \text{ sand} = \frac{2}{1+4} \times 4 \\ = 1.6\text{m}^3$$

(Lime stone is $18\text{kg}/\text{m}^2$),

$$(3) \text{ lime} = 1.8 \times 100 = 180\text{kg}$$

SNO	Des	Qty	Rate		Per	Amount	
			Rs	P ^s		Rs	P ^s
<u>Materials</u>							
(1)	Cement	12	120	00	per bags	1440	00
(2)	sand	1.6	300	00	per m ³	480	00
(3)	Lime	180	2	00	per kg	360	00
<u>Labours</u>							
	H.M	1/2 No	150	00	Per day	75	00
	M	10 Nos	100	00	— " —	1000	00
	Mazdoor	15 No)			— " —		
(1)	M	5 NO	70	00	— " —	350	00
(2)	F	10 No.	50	00	— " —	500	00
<u>Form Lime</u>							
	M ^a	10 Nos	100	00	— " —	1000	00
	C	20 Nos	50	00	— " —	1000	00
	Bhisti	1 Nos	60	00	— " —	60	00
		Sundant			Add 4(1/2)%	250	60
					w.c (2 1/2)%	125	30
					Add (10)%	626	10
						<u>7267</u>	<u>40</u>

Rs = 73 / m³

(10) Preparation of rate analysis for painting in C.M (1:2)

solⁿ:- considered 100m²

Materials

c.M required = $0.7 \text{ m}^3 / 100 \text{ m}^2$

cement = $\frac{0.7}{3} \times 1$

= 7 bags.

sand = $\frac{0.7 \times 2}{3}$

= 0.46 m^3 .

SNO	Description	Qty	Rate.		per unit	Amount	
			Rs	Amount		Rs	Amount
(1)	<u>Materials</u>						
	cement	7	120	00	per bag	840	00
	sand	0.46	300	00	per m ³	138	00
	<u>Labour</u>						
	H.M	1/2 NO	150	00	per day	75	00
	M	10 NO	100	00	"	1000	00
	<u>Maedoor</u>						
	M	4 NO	700	00	"	280	00
	F	6 NO	50	00	"	300	00
	Bhisti	1 NO	60			60	00
	sundaries (Add 1%)					27	00
						206	00
	Add					5282	00
	(2%) water chas					2720	00
	C.P (10%)					54	40
						272	00

$$\text{GRAND TOTAL} = 3046.4$$

$$\therefore \text{Rate of painting /m}^2 = \frac{3046.4}{100}$$

$$\text{Rs} = 30.46$$

$$\approx \text{30/-}$$

$$\approx 30.50/-$$

APPROXIMATE ESTIMATE

(Q) What is an approximate estimate

Ans) It is a rough estimate prepared to know the approx cost of a work in very short time. This type of estimate are prepared on the basis of actual cost of similar existing structure. It required administrative approval of a project.

Use of approx estimate

- (1) To give rough idea of probable expenditure.
- (2) To obtain administrative approval of a new project.
- (3) Used for valuation & rent fixation.

Methods of approx estimate?

- (1) service unit method
- (2) cubical content ———
- (3) Approx quantity ———
- (4) Plinth area ———
- (5) Typical bay ———

Service area method:

Every building is constructed to serve some purpose. Eg:- A hospital building is designed to accommodate certain no. of beds each bed is considered as a service unit of hospital building.

This method is particularly used in large or complicated buildings. Whenever a building is constructed, the engineer keeps the record of place of construction, year of construction, No of service units, actual cost of construction etc. The approx cost is found out by comparing the cost of proposed building with similar existing building.

$$\therefore \text{Approx cost of building} = \left[\begin{array}{l} \text{No of service unit} \\ \text{in proposed bldg} \end{array} \right] \times \left[\begin{array}{l} \text{cost per service} \\ \text{unit of existing} \\ \text{bldg} \end{array} \right]$$

est Plinth area method:-

This method also known as square metre method. In this method the plinth area of proposed bldg is calculated & approx cost of bldg is worked out on the basis of cost of similar existing bldg.

$$\therefore \text{Approx cost of bldg} = \left[\begin{array}{l} \text{Total plinth area} \\ \text{of proposed bldg} \end{array} \right] \times \left[\begin{array}{l} \text{cost/unit area} \\ \text{of similar} \\ \text{existing bldg} \end{array} \right]$$

For better result by this method following pt should be taken into consideration.

- (1) Ht of bldg.
- (2) Perimeter of bldg
- (3) Price index
- (4) Locatn of bldg.
- (5) Specificatn.
- (6) opening in bldg.

Cubical content method:-

This method of approx estimate is more accurate than plinth area method. This cubic content method of proposed bldg is found out. There the rate per cubic meter of the existing bldg is found out.

Approx cost of proposed bldg

$$= \left[\begin{array}{l} \text{Total cubic} \\ \text{content} \end{array} \right] \times \left[\begin{array}{l} \text{cost/cubic meter of} \\ \text{similar existing bldg} \end{array} \right]$$

Following it should be taken into considered

- (1) Ht of bldg.
- (2) Price index.
- (3) Locatⁿ of bldg.
- (4) Specificatⁿ & type of constructⁿ.

Typical bay method:-

The space betⁿ centre to centre of two successive columns is called bay. This method is suitable for bldg which have several identical bay like factory shed, cycle stand etc.

A typical bay is selected & cost of each & every item of bay is work out at prevailing market rate.

This cost is then divided by length of bay to obtain the cost/running metre of bldg.

$$\text{Approx cost of proposed area} = \left[\begin{array}{l} \text{No of bays} \\ \text{of proposed} \\ \text{bldg} \end{array} \right] \times \left[\begin{array}{l} \text{cost of one bay} \end{array} \right]$$

- to be considered while prepared by bay method
- (1) Framed structure should be identical
 - (2) End correctn should be done for end bays due to end walls.

PREPARATION OF ~~WORK~~ APPROX ESTIMATE OF DIFF ITEMS

Approx estimate for Road or highway

The unit of calculation for approx estimate for a highway or road is per kilometer length.

To prepare a approx estimate of road, consider a typical one kilometer length of a similar exist road. Then calculate the rate of each component of road and at std market rate. The approx rate of proposed road can be found by multiplying the total length of proposed road with rate/km for existing road.

The main component of road are

- (1) Land acquisitn
- (2) Earth work
- (3) Bridge, cd work (or) surface dressing
- (4) Road amenities

The approx estimate is also improved by making necessary adjustment in proposed & existing road by considering following factors

- (1) site conditn
- (2) price index
- (3) Type of cd work
- (4) Type of masonry.

Approx estimates for water supply projects

It consist of following steps

(I) collection of data:-

- (a) situation a boat of stream (e) Existing water supply scheme
- (b) configuration of river (f) Existing drainage
- (c) Rainfall data of that region (g) Necessity of the water scheme
- (d) population growth.

(II) Estimating the cost of project:-

To cost of the proposed water supply scheme the entire scheme is divided into following component a cost of each component is determine separately. The total cost is found out by adding cost of each component. To this cost add cost for work charge establishment. tool & plant. The final total runs obtained will be approximate of water supply scheme.

component of water supply scheme are

- 1) Preliminary survey (5) service reservoir
- 2) Head work (6) Treatment plant
- 3) Pump house (7) Distributn system
- 4) Rising mains (8) Lateral aqueduct
- Approach road

Head work:- (a) Dam

(b) In Alluvial gallery,

approx estimate for sanitary project:

First we have to divide it into components then the cost of each component is found out. To this about 20% total cost is added as work charge establishment. The grand total thus obtained give the rough estimate of sanitary project. The main components are

- (1) Preliminary survey
- (2) Collection media
- (3) Rising main
- (4) Sewage treatment plant
- (5) Land acquisition

Rough estimate for bridge or culvert

The approx cost of a bridge can be worked out by

- (1) The linear water way at the time of H.F. is found.
- (2) To find the rate of construction per m of linear water way of similar bridge the cost of construction is divided by linear water way.
- (3) To get the cost of proposed structure multiply the calculator rate proposed span to get the approx cost of structure.

Approx estimate for irrigation work

It is found out by one of the following method

- (a) Hectare meter of storage is considered the unit to arrive at the approx the quantity of water collected in hectare area of one metre depth is the

Hectare meter storage. The cost per hectare meter of similar existing structure is determined. The rate so obtained is then multiplied with total storage of proposed structure will be given the approx cost required.

(b) The complete irrigation project is divided into no. of units

- (1) Preliminary survey (3) Earthwork
 - (2) Masonry work (4) Land acquisition
- (The cost of each item is worked out individually by comparison with similar existing scheme. Add 20% W.C. establishment to the cost. This is an approx estimate of an irrigation scheme.)

Approx estimate for dam

The procedure are as follows

- (1) selectn of site for dam with help of topo sheet
- (2) calculate the C.A for proposed dam
- (3) Fix the capacity of dam by contours
- (4) collect the rainfall data at raingaugestation near the proposed C.A
- (5) calculate the constructn cost per million cumet
- (6) Multiply this cost with total storage capacity to this cost add 20% work charges (W.C) establishment. The grand total thus obtain is the approx cost of a dam.

Approx cost of a R.C.C retaining wall.

The approx cost of R.C.C retaining wall can be determined by knowing the total quantity of concrete used. The quantity will depend on the type of foundation available, type of back filling and nature of loading. Usually quantity per linear measurement is worked out then, the cost is corresponding determined. This cost is then multiplied by total length of the R.C.C retaining wall to get the approx cost of R.C.C retaining wall.

Method of approx costing for culvert

Depending upon the nature of foundation the type of material used, width of roadway etc the approx cost /m of span is estimated to work out the approx cost of a culvert whose span are known.

For culvert with small spans. the approx cost can be found out on lump sum basis per m from the record of culvert having smaller span.